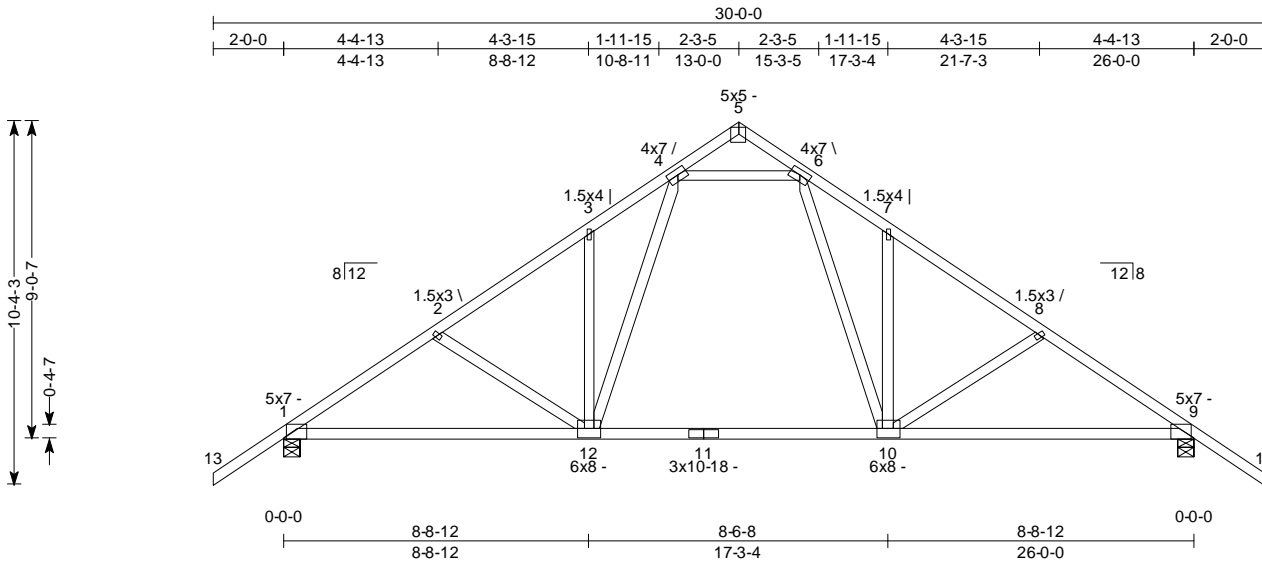


SPAN 26-0-0	PITCH 8/12	QTY 20	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 115 lbs	BRD FT/PLY 73.3
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Loading	General	CSI Summary	Deflection	L/	(loc)	Allowed
TCLL: 20 psf Snow: 38.1 psf TCDL: 10 psf(rake) BCLL: 0 psf BCDL: 10 psf	Bldg Code: IRC 2006/ TPI 1-2002 Rep Mbr Increase: Yes D.O.L.: 100 % Matrix	TC: 0.80 (3-4) BC: 0.59 (12-1) Web: 0.55 (3-12)	Vert TL: 0.35 in Vert LL: 0.24 in Horz TL: 0.21 in	L / 853 L / 999	(9-10) (10-11) 4	L / 240 L / 360
Plate Offsets (Jnt:X,Y,Ang): (1:4-5,1-15,0.) (2:0-0,4-3,32.) (3:0-0,4-3,90.) (4:6-5,0-0,34.) (5:0-0,4-3,0.) (6:6-5,0-0,34.) (7:0-0,4-3,90.) (8:0-0,4-3,32.) (9:4-5,1-15,0.) (10:0-0,3-8,0.) (11:0-0,1-12,0.) (12:0-0,3-8,0.)						

### Reaction Summary

JT	Type	Brg Combo	Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	Pin (Wall)	1	5.5 in	1,932 lbs	.	.	-145 lbs	-145 lbs	76 lbs
9	H Roll (Wall)	1	5.5 in	1,932 lbs	.	.	-145 lbs	-145 lbs	.

### Material Summary

TC	SPF 2100/1.8	2 x 4
BC	SPF 2100/1.8	2 x 4
Webs	SPF #2	2 x 4

### Bracing Summary

TC Bracing:	Sheathed or purlins at 3-9-0, Purlin design by Others.
BC Bracing:	Sheathed or purlins at 10-0-0, Purlin design by Others.

### Loads Summary

- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, Not End Zone Truss, Neither end web considered. DOL = 1.33
- This truss has been designed for the effects of balanced (8/12, 38.1 psf) and unbalanced (8/12, 11.4 psf wind, 38.1 psf lee, 38 psf lee over peak to 7 ft) snow loads for hips/gables in accordance with ASCE7 - 05 with the following user defined input: 60 psf ground snow load, Terrain Category C, Partially Exposed, Building Category II (I = 1.0), Ct = 1.00, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2006 assuming slope = 8/12 and area supported = 60 ft<sup>2</sup>.
- Minimum storage attic loading has been applied in accordance with IRC 301.5

### Load Case Lr1: Std Live Load

#### Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Bot	8-8-12	17-3-4	Down	Proj	10 psf	10 psf	24 in

### Member Forces Summary

Table indicates: Member ID, max CSI/Stress, max axial force, (max compr. force if different from max axial force)													
TC	13-1	0.559	213 lbs	3-4	0.800	-2,400 lbs	6-7	0.800	-2,400 lbs	9-14	0.559	213 lbs	
	1-2	0.533	-2,656 lbs	4-5	0.786	-331 lbs	7-8	0.409	-2,260 lbs				
	2-3	0.409	-2,260 lbs	5-6	0.786	-331 lbs	8-9	0.533	-2,656 lbs				
BC	9-10	0.590	2,130 lbs	10-12	0.564	1,344 lbs	12-1	0.590	2,130 lbs	(-107 lbs)			
Webs	2-12	0.216	-442 lbs	4-12	0.328	1,339 lbs	6-10	0.328	1,339 lbs	(-49 lbs)	8-10	0.216	-442 lbs
	3-12	0.545	-906 lbs	4-6	0.329	-1,228 lbs	7-10	0.545	-906 lbs				

### Notes:

- When this truss has been chosen for quality assurance inspection, the Plate Placement Method per TPI 1-2002/A3.2 shall be used.
- Brace bottom chord with properly applied gypsum board or approved equal, unless noted otherwise.