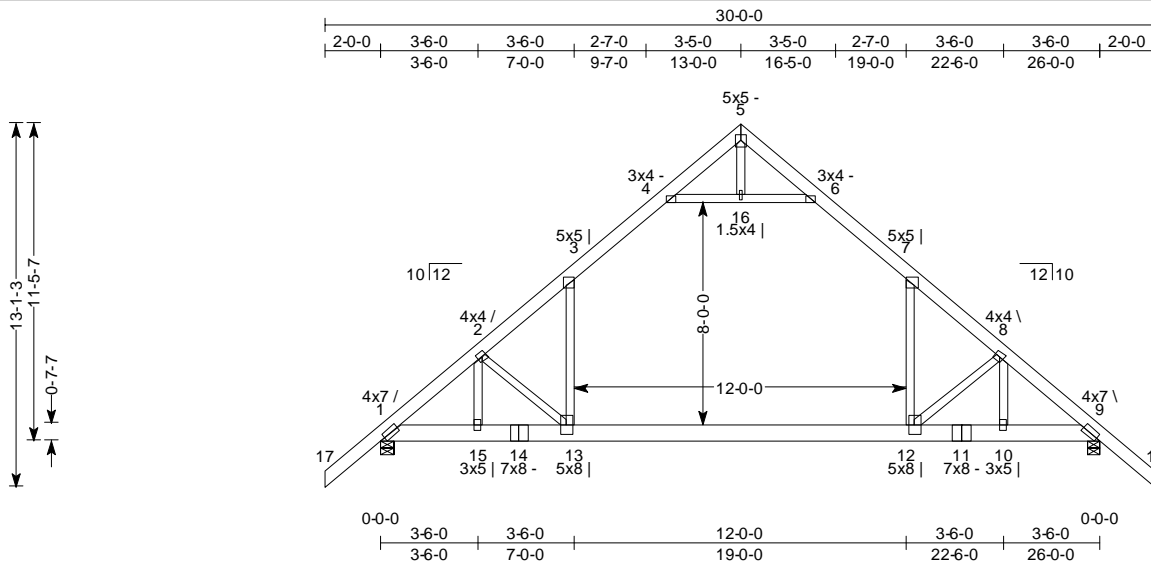


SPAN 26-0-0	PITCH 10/12	QTY 20	OHL 2-0-0	OHR 2-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 199 lbs	BRD FT/PLY 97.3
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Loading	General	CSI Summary	Deflection	L/	(loc)	Allowed
TCLL : 20 psf Snow : 31.7 psf TCDL : 10 psf(rake) BCLL : 0 psf BCDL : 10 psf	Bldg Code : IRC 2006/ TPI 1-2002 Rep Mbr Increase : Yes D.O.L. : 100 % Matrix	TC : 0.83 (2-3) BC : 0.52 (12-13) Web : 0.42 (6-16)	Vert TL: 0.75 in Vert LL: 0.42 in Horz TL: 0.02 in	L / 402 L / 724	(12-13) (12-13) 9	L / 240 L / 360

Plate Offsets (Jnt:X,Y,Ang): (1:4-3.3-10,40.) (2:1-12.5-11,40.) (3:2-8.8-10,90.) (4:10-11.1-12,0.) (5:0-0.7-3,0.) (6:10-11.1-12,0.) (7:2-8.8-10,90.) (8:1-12.5-11,40.) (9:4-3.3-10,40.) (10:0-0.7-4,90.) (11:0-0.3-10,0.) (12:3-8.7-4,90.) (13:3-8.7-4,90.) (14:0-0.3-10,0.) (15:0-0.7-4,90.) (16:0-0.3-8,90.)

Reaction Summary

JT	Type	Brg Combo	Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	Pin (Wall)	1	5.5 in	2,130 lbs	128 lbs
9	H Roll (Wall)	1	5.5 in	2,130 lbs

Material Summary

TC	SP 2700/2.2	2 x 6
BC	SP 2400/2.0	2 x 8
Webs	SPF #2	2 x 4

Bracing Summary

TC Bracing:	Sheathed or purlins at 4-6-0, Purlin design by Others.
BC Bracing:	Sheathed or purlins at 10-0-0, Purlin design by Others.

Loads Summary

- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, Not End Zone Truss, Neither end web considered. DOL = 1.33
- This truss has been designed for the effects of balanced (10/12, 31.7 psf) and unbalanced (10/12, 9.5 psf wind, 31.7 psf lee, 42.5 psf lee over peak to 6.2 ft) snow loads for hips/gables in accordance with ASCE7 - 05 with the following user defined input: 60 psf ground snow load, Terrain Category C, Partially Exposed, Building Category II (I = 1.0), Ct = 1.00, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of a 16 psf live load computed in accordance with IRC 2006 assuming slope = 10/12 and area supported = 60 ft^2.
- Minimum storage attic loading has been applied in accordance with IRC 301.5

Member Forces Summary

Table indicates: Member ID, max CSI/Stress, max axial force, (max compr. force if different from max axial force)

TC	17-1	0.184	248 lbs	3-4	0.832	-1,790 lbs	6-7	0.832	-1,790 lbs	9-18	0.184	248 lbs
	1-2	0.186	-2,857 lbs	4-5	0.751	399 lbs	7-8	0.835	-2,686 lbs			
	2-3	0.835	-2,687 lbs	5-6	0.751	399 lbs	8-9	0.186	-2,857 lbs			
BC	9-10	0.154	2,230 lbs	12-13	0.516	1,695 lbs	15-1	0.154	2,230 lbs			
	10-12	0.422	2,230 lbs	13-15	0.422	2,230 lbs						
Webs	2-15	0.089	-537 lbs	4-16	0.420	-2,112 lbs	7-12	0.384	1,417 lbs			
	2-13	0.247	-765 lbs	5-16	0.015	56 lbs	8-12	0.247	-765 lbs			
	3-13	0.384	1,417 lbs	6-16	0.420	-2,112 lbs	8-10	0.088	-537 lbs			

Notes:

- Attic floor area has been designed as a living area with 40 psf floor live load and a 10 psf floor dead load, and the interior vertical webs and ceiling has been designed for a 5 psf dead load.
- When this truss has been chosen for quality assurance inspection, the Plate Placement Method per TPI 1-2002/A3.2 shall be used.
- Brace bottom chord with properly applied gypsum board or approved equal, unless noted otherwise.
- At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.